

ICC-ES Evaluation Report

ESR-1663

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This report also contains:

Revised March 27, 2025

- City of LA Supplement

Subject to renewal March 2027 - FL Supplement w/ HVHZ

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Fasteners

REPORT HOLDER: EVALUATION SUBJECT:

HILTI LOW-VELOCITY POWER-ACTUATED

FASTENERS



1.0 EVALUATION SCOPE

Compliance with the following codes:

- 2024, 2021, 2018 and 2015 *International Building Code*® (IBC)
- 2024, 2021, 2018 and 2015 International Residential Code® (IRC)

Section number references in this report are for the 2024 IBC and IRC and the standards referenced therein. Corresponding section numbers for earlier code editions are shown in <u>Table 8</u> at the end of this report.

Property evaluated:

Structural

2.0 USES

Hilti low-velocity power-actuated fasteners (PAFs) are used to attach wood, cold-formed steel, and other building elements to base materials of normalweight and sand-lightweight concrete, steel deck panels filled with sand-lightweight concrete, concrete masonry and steel base materials. The fasteners are alternatives to the cast-in-place anchors described in IBC Section 1901.3 for placement in concrete; the embedded anchors described in Section 8.1.4 of TMS 402, referenced in IBC Section 2107 for placement in masonry; and the steel connections described in IBC Section 2201.4. For structures regulated under the IRC, the fasteners may be used where an engineered design is submitted in accordance with IRC Section R301.1.3.

The PAFs are intended for use in redundant applications, which are defined as applications where multiple PAFs support elements that are capable of redistributing the load to neighboring PAFs, in the event of a PAF failure. Examples include, but are not limited to, PAFs used to fasten cold-formed steel track, where the track can redistribute loads; and PAF assemblies where the supported elements, such as conduit and ceiling framing can redistribute the loads to neighboring PAF assemblies.

3.0 DESCRIPTION

3.1 Fasteners:

The Hilti low-velocity power-actuated fasteners are manufactured from hardened steel complying with the material specifications in the manufacturer's quality documentation. See Table 1 for fastener descriptions, including shank type and diameter, head diameter, coating and applicable allowable load tables. See Figure 1 for fastener images. Maximum point length is the maximum specified length from the tip of the fastener to the location where the diameter of the shank becomes constant. Minimum effective shank length is the minimum specified length from the underside of the fastener head to the tip of the fastener, except for fasteners with premounted washers, where the minimum effective shank length is the minimum specified length from the underside of the washer, in its installed condition, to the tip of the fastener.

3.2 Substrate Materials:

- **3.2.1 Concrete:** Normalweight and sand-lightweight concrete must comply with IBC Chapter 19 or IRC Section R402.2, as applicable. The minimum concrete compressive strength at the time of fastener installation must be as noted in <u>Table 4</u>.
- **3.2.2 Concrete Masonry:** Concrete masonry units (CMUs) must be minimum 8-inch-thick (203 mm), normalweight blocks complying with ASTM C90. Mortar must comply with ASTM C270, Type N. Grout must be coarse grout complying with ASTM C476. Concrete masonry walls must have a minimum compressive strength, f'_m , of 1,500 psi (10.3 MPa).
- **3.2.3 Steel:** Structural steel used in supports must comply with the minimum requirements of ASTM A36, ASTM A572 Grade 50 or ASTM A992, as applicable, and must have the minimum yield and tensile strengths and thickness shown in <u>Table 2</u> or <u>3</u>, as applicable.
- **3.2.4 Steel Deck Panels:** Steel deck panel properties and configurations must be as described in <u>Tables 5</u> and <u>6</u> and <u>Figures 2</u> through <u>4</u>.

4.0 DESIGN AND INSTALLATION

4.1 Design:

4.1.1 General: Design of the connection of attached material to the base material must take into account the properties of the attached material and the need for redundancy, and must comply with the applicable requirements of the IBC.

Selection of fasteners must take into consideration the applicable base material and the length of the fastener. The minimum fastener length must be determined as follows:

- For installation into concrete, concrete-filled steel deck panels and concrete masonry materials and into
 steel base materials where there is no point penetration, the minimum effective shank length shown in
 Table 1 must equal or exceed the sum of the thickness of the attached material and the minimum
 embedment depth (penetration) shown in the applicable tables in this report.
- For installation through steel base material, the minimum effective shank length shown in <u>Table 1</u> must equal or exceed the sum of the following: the thickness of the attached material, the thickness of the base material and the required point penetration shown in the applicable tables in this report.
- **4.1.2 Allowable Loads:** The applicable allowable load tables for Hilti power-actuated fasteners driven into different base materials may be determined by referencing <u>Table 1.</u>

The most critical applied loads, excluding seismic load effects, resulting from the load combinations in Section 2.4 of ASCE 7 (referenced in IBC Section 1605.1) or IBC Section 1605.2 must not exceed the allowable loads. For fasteners which are subjected to seismic loads, see Section 4.1.5 for additional information. The stress increases and load reductions described in IBC Section 1605.2 are not allowed.

Allowable shear loads and tension (pullout) loads in this report apply to the connection of the fastener to the base material only. Other limit states applicable to the design of a connection, such as fastener pull-through (pull-over) and lateral bearing on the attached material, which are governed by the properties of the attached material, are outside the scope of this report. Design of the connection to the attached material must comply with the applicable requirements of the IBC. When designing the connection of wood members to the base material, the bending yield strength of the PAFs can be assumed to be the same as that of a nail with the same shank diameter.

4.1.3 Combined Loading: For fasteners subjected to both tension and shear loads, compliance with the following interaction equation must be verified:

$$(p/P_a) + (v/V_a) \le 1.0$$

where:

p = Actual tension load on the fastener, lbf (N).

 P_a = Allowable tension load for the fastener, lbf (N).

v = Actual shear load on the fastener, lbf (N).

 V_a = Allowable shear load for the fastener, lbf (N).

- **4.1.4** Steel-to-steel Connections: When the Hilti fasteners listed in <u>Tables 2</u> and <u>3</u> are used in connections of two steel elements in accordance with Section J5 of AISI S100, connection capacity must be determined in accordance with Sections 4.1.4.1 and 4.1.4.2, as applicable.
- **4.1.4.1 Connection Strength Tension:** To determine tensile connection strength in accordance with Section J5.2 of AISI S100, fastener tension strength, the pull-out strength and the pull-over strength must be known. These characteristics must be determined as follows:
 - Pull-out Strength: See Table 2 or 3 for available pull-out strength, as applicable.
 - Pull-over Strength: The available pull-over strengths must be calculated in accordance with Section J5.2.3 of AISI S100.
 - PAF Tensile Strength: The allowable fastener tension strengths, determined in accordance with Section J5.2.1 of AISI S100, exceed the corresponding allowable pull-out strengths in Tables 2 and 3.
- **4.1.4.2 Connection Strength Shear:** To determine shear connection strength in accordance with Section J5.3 of AISI S100, the fastener shear strength, bearing and tilting strength, pull-out strength in shear, net section rupture strength and shear strength limited by edge distance must be known. These characteristics must be determined as follows:
- Bearing and Tilting Strength: The available bearing and tilting strengths must be calculated in accordance with Section J5.3.2 of AISI S100.
- Pull-out Strength in Shear: The available pull-out strength in shear must be the applicable allowable shear strength from <u>Table 2</u> or <u>3</u>, as applicable, or must be calculated in accordance with Section J5.3.3 of AISI S100.
- Net Section Rupture Strength and Shear Strength Limited by Edge Distance: The net section rupture
 strength must be determined in accordance with Section J5.3.4 of AISI S100 and the shear strength
 limited by edge distance must be determined in accordance with Section J5.3.5 of AISI S100.
- PAF Shear Strength: The allowable fastener shear strengths, determined in accordance with Section J5.3.1 of AISI S100, exceed the corresponding allowable shear strengths in Tables 2 and 3.
- **4.1.5** Seismic Considerations: When subjected to seismic loads, the Hilti fasteners may be used as follows:
 - 1. The Hilti fasteners may be used for attachment of nonstructural components listed in Table 13.1-1 of ASCE 7, which are exempt from the requirements of ASCE 7.
 - Concrete base materials: The Hilti fasteners installed in concrete may be used to support acoustical tile
 or lay-in panel suspended ceiling systems, distributed systems and distribution systems where the
 service load on any individual fastener does not exceed the lesser of 90 lbf (400 N) or the published
 allowable load in Tables 4, 5 and 6, as applicable.
- 3. Steel base materials: When the Hilti fasteners listed in <u>Table 2</u> or <u>3 [except for the X-R fastener and the X-ENP-19 L15 fastener installed in ¹/₄-inch-thick (6.4 mm) steel] are installed in steel and subjected to seismic loads, the most critical load applied to each individual fastener must be determined from the applicable equations in Section 2.4 of ASCE 7 (referenced in 2021 IBC Section 1605.1) or IBC Section</u>

- 1605.2, and must not exceed the allowable load shown in <u>Table 2</u> or <u>3</u>, as applicable, taking into account the footnotes to the applicable table. The X-R fastener and the X-ENP-19 L15 fastener installed in ¹/₄-inch-thick (6.4 mm) steel may be used where the service load on any individual fastener does not exceed the lesser of 250 lbf (1112 N) or the published allowable load shown in <u>Table 2</u> or <u>3</u>, as applicable. Evaluation of the Hilti fasteners installed in steel base material for use in the design of lateral force resisting systems, such as shear walls and diaphragms, is outside the scope of this report.
- 4. For interior, nonstructural walls that are not subject to sustained tension loads and are not a bracing application, the power-actuated fasteners may be used to attach steel track to concrete or steel in all Seismic Design Categories. In Seismic Design Categories D, E, and F, the allowable shear load due to transverse pressure shall be no more than 90 pounds (400 N) when attaching to concrete; or the allowable load shown in Table 2 or 3, as applicable, when attaching to steel. Substantiating calculations shall be submitted addressing the fastener-to-base-material capacity and the fastener-to-attached-material capacity. Interior nonstructural walls are limited to locations where bearing walls, shear walls or braced walls are not required by the approved plans. The design load on the fastener must not exceed the allowable load established in this report for the concrete or steel base material.

4.2 Installation:

4.2.1 General: The fasteners must be installed in accordance with this report and the Hilti, Inc., published installation instructions. A copy of the instructions must be available on the jobsite at all times during installation. Additional installation requirements are set forth in the tables in this report.

Fastener placement requires a low-velocity powder-actuated tool used in accordance with Hilti, Inc. recommendations.

Installers must be certified by Hilti, Inc., and have a current, Hilti-issued, operator's license.

- **4.2.2 Fastening to Steel:** When installation is in steel, minimum spacing between fasteners must be 1 inch (25.4 mm) on center, and minimum edge distance must be $\frac{1}{2}$ inch (12.7 mm).
- **4.2.3 Fastening to Concrete:** Fasteners are to be driven into the concrete after the concrete attains the concrete strength specified in the tables of this report. Unless otherwise noted, minimum spacing between fasteners must be 4 inches (102 mm) on center and minimum edge distance must be 3 inches (76 mm). Unless otherwise noted, concrete thickness must be a minimum of three times the embedment depth of the fastener.
- **4.2.4 Fastening to Masonry:** Fasteners are to be driven into the masonry after the mortar and grout materials have attained the specified strength. For CMUs, no more than one power-actuated fastener may be installed into each individual CMU cell.
- **4.2.5** Fastening to Sand-lightweight Concrete-filled Steel Deck Panels: Installation in sand-lightweight concrete-filled steel deck panels must comply with <u>Tables 5</u> and <u>6</u> and <u>Figures 2</u> through <u>4</u>. Minimum distances from fastener centerline to rolled deck panel flute edges must be as depicted in <u>Figures 2</u> through <u>4</u>.
- **4.2.6 Use with Treated Lumber:** The Hilti carbon steel fasteners described in <u>Table 1</u> may be used in contact with fire-retardant-treated wood in dry, interior locations only, in accordance with IBC Section 2304.10.6.4 and Hilti's recommendations. Use of fasteners in contact with preservative-treated wood or fire-retardant-treated wood in exterior applications is outside the scope of this report

5.0 CONDITIONS OF USE:

The Hilti Low-Velocity Power-Actuated Fasteners described in this report comply with, or are suitable alternatives to what is specified in, those codes listed in Section 1.0 of this report, subject to the following conditions:

- 5.1 Fasteners must be manufactured and identified in accordance with this report.
- **5.2** Fasteners must be installed in accordance with this report and the Hilti, Inc., instructions. In the event of conflict between this report and Hilti, Inc., published instructions, the more restrictive requirements govern.
- **5.3** Calculations demonstrating that the actual loads are less than the allowable loads described in Section 4.1 must be submitted to the code official for approval. The calculations must be prepared by a registered design professional where required by the statutes of the jurisdiction in which the project is constructed.
- 5.4 For steel-to-steel connections that meet the applicability requirements of Section J5 of AISI S100, calculations demonstrating that the available connection strength has been determined in accordance with Section J5 of AISI S100 and Section 4.1.4 of this report, and equals to or exceeds the applied load, must be submitted to the code official. The calculations must be prepared by a registered design professional where required by the statutes of the jurisdiction in which the project is to be constructed.

- **5.5** Refer to Section 4.1.5 for seismic considerations.
- **5.6** Diaphragm design values using the X-ENP-19 L15 fasteners must be determined in accordance with ESR-2197 and ESR-2776, as applicable.
- **5.7** The use of the fasteners is limited to installations in uncracked concrete or masonry. Cracking occurs when $f_t > f_r$ due to service loads or deformations.
- **5.8** Hilti X-CR and X-R stainless steel fasteners may be used in exterior, damp environments. All other fasteners in this report must be limited to installation in dry, interior environments, which include exterior walls which are protected by an exterior wall envelope.
- **5.9** Installation must comply with Section 4.2.6 regarding fasteners in contact with preservative-treated and fire-retardant-treated wood.
- 5.10 Installers must be certified by Hilti, Inc., and have a current, Hilti-issued, operator's license.
- **5.11** The Hilti products addressed in this report are manufactured under a quality-control program with inspections by ICC-ES.

6.0 EVIDENCE SUBMITTED

Data in accordance with the ICC-ES Acceptance Criteria for Power-actuated Fasteners Driven into Concrete, Steel, and Masonry Elements (AC70), dated July 2024, including seismic load test data in accordance with Annex A of AC70.

7.0 IDENTIFICATION

- **7.1** The ICC-ES mark of conformity, electronic labeling, or the evaluation report number (ICC-ES ESR-1663) along with the name, registered trademark, or registered logo of the report holder must be included in the product label.
- 7.1 In addition, all carbon steel fasteners are identified by an "H" imprinted on the fastener head. The stainless steel fasteners are identified by "HI" imprinted on the fastener head. Where applicable, the word "Hilti" is stamped on the steel washers. All fasteners are packaged in containers noting the fastener type and size.
- **7.2** The report holder's contact information is the following:

HILTI, INC. 7250 DALLAS PARKWAY, SUITE 1000 PLANO, TEXAS 75024 (800) 879-8000 www.hilti.com/

TABLE 1—FASTENER DESCRIPTION AND APPLICATIONS¹

FASTENER	FASTENER DESCRIPTION	SHANK TYPE	SHANK DIAMETER (inch)	HEAD DIAMETER (inch)	MAXIMUM POINT LENGTH (inch)	MINIMUM EFFECTIVE SHANK LENGTH (inch)	FASTENER MATERIAL/ COATING	APPLICABLE BASE MATERIAL	APPLICABLE LOAD TABLES
X-ENP-19 L15	Powder-actuated Heavy Duty Fastener	Knurled, tapered	0.177	0.291	0.56	0.748		Steel	<u>2</u>
EDS ##	Powder-actuated Heavy Duty Fastener	Knurled, straight	0.177	0.390	0.43	See Footnote 2		Steel	<u>2</u>
DS ##	Powder-actuated Heavy Duty Fastener	Smooth, straight	0.177	0.390	0.43	See Footnote 2		Steel Concrete Concfilled deck	<u>2</u> <u>4, 5</u>
X-C ##	Powder-actuated Standard Fastener	Knurled, straight	0.138	0.321	0.30	See Footnote 2	Carbon steel	Concrete Concfilled deck	
X-C22P8TH	Powder-actuated Standard Fastener	Knurled, straight	0.138	0.321	0.30	0.807	ASTM B633, SC1, Type III	CMU Concrete Concfilled deck	<u>7</u> <u>4, 5, 6</u>
X-C20THP	Powder-actuated Standard Fastener	Knurled, straight	0.138	0.321	0.30	0.728		Concrete Concfilled deck	<u>5</u>
X-W6	Powder-actuated 1/4-20 Threaded Stud	Smooth, straight	0.145	n/a	n/a	n/a		Concrete Concfilled deck	<u>4</u> , <u>5</u>
W10	Powder-actuated ³ / ₈ -16 Threaded Stud	Smooth, straight	0.205	n/a	n/a	n/a		CMU Concrete Concfilled deck	<u>7</u> <u>4, 5</u>
	Powder-actuated	0 1						Steel	<u>3</u>
X-CR ##	Stainless Steel Fastener	Smooth, straight	0.145	0.321	0.43	See Footnote 2	Stainless steel	Concrete Concfilled deck	<u>4, 5</u>
X-R	Powder-actuated Stainless Steel Fastener	Smooth, straight	0.145	0.321	0.35	0.531	Stainless steel	Steel	<u>3</u>

For **SI:** 1 inch = 25.4 mm.

TABLE 2—ALLOWABLE LOADS FOR CARBON-STEEL LOW-VELOCITY FASTENERS DRIVEN INTO STEEL 1.2.3

FASTENER DESCRIPTION	FASTENER	SHANK DIAMETER (inch)	ALLOWABLE LOADS (lbf)									
Steel Thickness (inch):			3/	16	1,	1 ₄	3/	1 8	1,	/ ₂	3/	4
Loa	Load Direction:		Tension	Shear	Tension	Shear	Tension	Shear	Tension	Shear	Tension	Shear
Heavy Duty Knurled Shank	X-ENP-19 L15 ⁽⁴⁾	0.177	-	-	710 ⁽⁶⁾	715 ⁽⁶⁾	790 ⁽⁷⁾	775	735 ⁽⁸⁾	720 ⁽⁹⁾	735	720
Heavy Duty Knurled Shank	EDS	0.177	305(5)	615	625(5)	870	715 ⁽⁵⁾	870	890(5)	960	-	_
Heavy Duty Smooth Shank	DS	0.177	365(5)	725	580 ⁽⁵⁾	725	695 ⁽⁵⁾	725	735	860	_	-

For **SI**: 1 inch = 25.4 mm, 1 lbf = 4.45 N, 1 ksi = 6.89 MPa.

^{1##} denotes numbers used in fastener designation to represent nominal fastener length in mm.

²When multiple lengths of a fastener are addressed, the minimum effective shank length can be calculated in terms of the designated length as (##-1) in mm and (##-1)/25.4 in inches.

¹Fasteners must be driven to where the point of the fastener fully penetrates through the steel base material, unless otherwise noted.

²Allowable load capacities are based on base steel with a minimum yield strength (F_y) of 36 ksi and a minimum tensile strength (F_u) of 58 ksi.

³Unless otherwise noted, allowable loads are applicable to static and seismic loads in accordance with Section 4.1.

⁴Values are based on installation through metal sheet material with a maximum thickness of 0.1345 inch. Point penetration is not necessary.

⁵For steel-to-steel connections designed in accordance with Section 4.1.4, the tabulated allowable load may be increased by a factor of 1.25, and the design strength may be taken as the tabulated allowable load multiplied by a factor of 2.0.

⁶Tabulated allowable load applies to static load conditions only. For seismic loading, allowable loads must be limited in accordance with Section 4.1.5, Item 3

⁷Tabulated allowable load is applicable to static load conditions only. For seismic loading, the allowable load must be limited to the tabulated allowable static load multiplied by 0.80.

⁸Tabulated allowable load is applicable to static load conditions only. For seismic loads, the allowable load must be limited to the tabulated allowable static load multiplied by 0.68.

⁹Tabulated allowable load is applicable to static load conditions only. For seismic loads, the allowable load must be limited to the tabulated allowable static load multiplied by 0.95.

TABLE 3—ALLOWABLE LOADS FOR STAINLESS STEEL LOW-VELOCITY FASTENERS DRIVEN INTO STEEL 1,2,3

FASTENER DESCRIPTION	FASTENER	SHANK DIAMETER (inch)	ALLOWABLE LOADS (lbf)									
Steel T	Steel Thickness (inch):			¹ / ₈		³ / ₁₆		1/4		³/ ₈		2
Loa	Load Direction:		Tension	Shear	Tension	Shear	Tension	Shear	Tension	Shear	Tension	Shear
Stainless Steel Smooth Shank	X-CR X-R	0.145	_	_	460 ⁽⁸⁾	460	615 ⁽⁸⁾	500	_	_	_	_
Stainless Steel Smooth Shank	X-CR ^{4,9} X-R ^{4,9}	0.145	300 ⁽⁵⁾	190	615 ⁽⁸⁾	495	760 ⁽⁸⁾	500	220(5)	325 ⁽⁶⁾	225 ⁽⁷⁾	335 ⁽⁷⁾

For **SI:** 1 inch = 25.4 mm, 1 lbf = 4.4 N, 1 ksi = 6.89 MPa.

TABLE 4—ALLOWABLE LOADS FOR LOW-VELOCITY FASTENERS DRIVEN INTO NORMAL-WEIGHT CONCRETE^{1,2}

FASTENER DESCRIPTION	FASTENER	SHANK DIAMETER (inch)	MINIMUM EMBEDMENT DEPTH (inches)	ALLOWABLE LOADS (lbf)					
Co	ncrete Compressi	ve Strength:		2,500	0 psi	4,000	psi		
	Load Direction:				Shear (lbf)	Tension (lbf)	Shear (lbf)	Tension (lbf)	Shear (lbf)
			3/4	45	75	65	105	95	195
Standard Nail	X-C	0.138	1	85	150	160	200	105	270
Standard Ivali	X-C		1 ¹ / ₄	130	210	270	290	165	325
			11/2	175	260	270	360	_	_
Drywall Track Nail	X-C22 P8 TH	0.138	3/4	55	130	90	170	100	200
	DS	0.177	3/4	50	120	125	135	_	_
Haara Dutu Nail			1	130	195	155	240	_	_
Heavy Duty Nail			1 ¹ / ₄	220	385	270	425	_	_
			11/2	300	405	355	450	_	_
1/4-20 Threaded	X-W6	0.145	3/4	40	55	40	55	_	_
Stud	X-VVO	0.145	1	85	195	110	225	_	_
			1	85	95	100	105	_	_
³ / ₈ -16 Threaded Stud	W10	0.205	1 ¹ / ₄	175	345	200	380	_	_
Cida			1 ⁵ / ₈	285	380	385	395	_	_
			3/4	30	40	65	40	_	_
Stainless Steel	V 0D	0.145	1	55	185	120	190	100	170
Nail	X-CR	0.145	1 ¹ / ₄	110	290	125	300	120	440
			11/2	265	405	350	450	_	_

For **SI:** 1 inch = 25.4 mm, 1 psi = 6.89 kPa, 1 lbf = 4.45 N.

¹Fasteners must be driven to where the point of the fastener fully penetrates through the steel base material, unless otherwise noted.

²Unless otherwise noted, allowable load capacities are based on base steel with a minimum yield strength (F_y) of 36 ksi and a minimum tensile strength (F_z) of 58 ksi.

³Unless otherwise noted, allowable loads are applicable to static and seismic loads in accordance with Section 4.1.

⁴Allowable load capacity based on base steel with minimum yield strength (F_y) of 50 ksi and minimum tensile strength (F_u) of 65 ksi.

⁵The fastener must penetrate through the steel, but full fastener point penetration through the steel is not necessary.

⁶Fastener point penetration through the steel is not necessary, provided a minimum embedment depth of 0.354 inch is achieved.

⁷Fastener point penetration through the steel is not necessary, provided a minimum embedment depth of 0.470 inch is achieved.

⁸For steel-to-steel connections designed in accordance with Section 4.1.4, the tabulated allowable load may be increased by a factor of 1.25, and the design strength may be taken as the tabulated allowable load multiplied by a factor of 2.0.

⁹Tabulated loads apply to static load conditions only. For seismic loading, allowable loads must be limited in accordance with Section 4.1.5, Item 3.

¹Fasteners must not be driven until the concrete has reached the designated minimum compressive strength.

²The fasteners listed in the table above may be used for static load conditions and for the seismic load conditions described in Section 4.1.5, as applicable. The tabulated allowable loads apply to static load conditions. For seismic load conditions, the allowable loads must be limited in accordance with Section 4.1.5, Items 2 and 4, as applicable.

TABLE 5—ALLOWABLE LOADS FOR LOW-VELOCITY FASTENERS DRIVEN INTO MINIMUM f_c =3,000 psi SAND-LIGHTWEIGHT CONCRETE^{1,4}

FASTENER DESCRIPTION	FASTENER	SHANK DIAMETER (inch)	MINIMUM EMBEDMENT DEPTH (inches)	ALLOWABLE LOADS (lbf)						
	Fastener L	ocation:		Installed in	to Concrete	Installed Through Steel Deck Panel into Concrete ^{2,3}				
			Upper Flute Lower Flute							
Load Direction:				Tension	Shear	Tension	Tension	Shear		
		0.138	3/4	120	175	120	95	265		
Standard Nail	V C		1	180	260	215	155	485		
Standard Ivali	X-C		11/4	225	400	250	200	500		
			11/2	285	400	285	210	555		
Drawell Trook Noil	X-C20 THP	0.138	⁵ / ₈	55	110	-	45	285		
Drywall Track Nail	X-C22 P8TH	0.138	3/4	120	220	120	95	260		
	DS⁵	0.177	3/4	100	200	-	-	200		
Heavy Duty Nail			1	180	360	-	180	405		
Heavy Duty Naii			11/4	300	520	-	-	515		
			11/2	450	680	-	325	625		
			1	230	240	-	-	240		
Stainless Steel Nail	X-CR	0.145	11/4	320	400	-	-	400		
			11/2	405	500	-	-	500		
1/4-20 Threaded	X-W6	0.145	3/4	125	185	125	115	185		
Stud	^-VVO	0.145	1	175	185	160	180	185		
24			1	265	185	-	-	185		
³ / ₈ -16 Threaded Stud	W10	0.205	11/4	280	380	160	210	685		
0.00			1 ⁵ / ₈	445	540	435	325	945		

For **SI:** 1 inch = 25.4 mm, 1 psi = 6.89 kPa, 1 lbf = 4.45 N.

¹Fasteners must not be driven until the concrete has reached the designated minimum compressive strength.

²The steel deck panel profile must be 3-inch-deep composite floor deck panel, with a minimum 0.0329-inch base-metal thickness, a minimum yield strength of 33 ksi and a minimum tensile strength of 45 ksi. Lower and upper flute widths must be a minimum of 3⁷/₈ inches. Figure 2 shows the nominal flute dimensions, fastener locations and load orientations for the deck panel profile.

³Sand-lightweight concrete fill depth above top of steel deck panel must be a minimum of 3¹/₄ inches.

⁴The fasteners listed in the table above may be used for static load conditions and for the seismic load conditions described in Section 4.1.5, as applicable. The tabulated allowable loads apply to static load conditions. For seismic load conditions, the allowable loads must be limited in accordance with Section 4.1.5, Items 2 and 4, as applicable.

⁵DS fasteners installed at 1 1/2-inch embedment through steel deck panel into the lower flute must be installed at a minimum distance of 6 inches from the edge of the floor deck panel.

TABLE 6—ALLOWABLE LOADS FOR LOW-VELOCITY FASTENERS DRIVEN INTO MINIMUM $f_c=3,000~{ m psi}$ SAND-LIGHTWEIGHT CONCRETE-FILLED 1 1 /2-INCH-DEEP, B-DECK STEEL PANEL 1,4

				ALLOWABLE LOADS (lbf) Installed Through Steel Deck Panel Into Concrete ^{2,3}			
FASTENER	FASTENER	SHANK DIAMETER	MINIMUM EMBEDMENT DEPTH				
DESCRIPTION		(inch)	(inch)	Upper Flute Lower Flu		Flute	
			, ,	Tension	Tension	Shear	
Drywall track nail	X-C22 P8 TH	0.138	3/4	90	110	295	
Standard nail	X-C	0.429	3/4	80	80	315	
		0.138	1	205	205	445	

For **SI:** 1 inch = 25.4 mm, 1 psi = 6.89 kPa, 1 lbf = 4.45 N.

TABLE 7—ALLOWABLE LOADS FOR LOW-VELOCITY FASTENERS DRIVEN INTO CONCRETE MASONRY UNITS^{1,2,3,9}

FASTENER DESCRIPTION	FASTENER	SHANK	EMBED- MENT DEPTH (inch)	ALLOWABLE LOADS (lbf)										
	Masonry Type:				Hollow CMU				Grouted CMU					
F	Fastener Location:			Face Shell ⁴ Mortar Joint ⁵			Face Shell⁴		Mortar Joint⁵		Top of Grouted Cell ⁸			
	Load Direc	tion:		Tension	Shear ⁶	Tension	Shear ⁶	Tension	Shear ⁶	Tension	Shear ⁷	Tension	Shear ⁶	
Standard Nail	X-C	0.138	1	40	85	15	50	85	85	45	85	115	175	
1/4-20 Threaded Stud	X-W6	0.145	1	105	175	80	110	125	175	135	150	_		

For **SI:** 1 inch = 25.4 mm, 1 lbf = 4.45 N.

¹Fasteners must not be driven until the concrete has reached the designated minimum compressive strength.

²Steel deck panel profiles are 1¹/₂-inch-deep, B-type deck panel with a minimum base-metal thickness of 0.0329 inch, and a minimum yield strength of 38 ksi and a minimum tensile strength of 45 ksi. Fasteners may be installed through steel deck panels having either normal or inverted orientations with minimum lower flute widths of 1³/₄ and 3¹/₂ inches, respectively. Fasteners must be placed at centerline of deck panel flutes. <u>Figures 3</u> and <u>4</u> describe additional flute dimensions, fastener locations, and load orientations for both deck panel profiles.

³Sand-lightweight concrete fill above top of steel deck panel must be a minimum of 2¹/₂ inches.

⁴The fasteners listed in the table above may be used for static load conditions and for the seismic load conditions described in Section 4.1.5, as applicable. The tabulated allowable loads apply to static load conditions. For seismic load conditions, the allowable loads must be limited in accordance with Section 4.1.5, Items 2 and 4, as applicable.

¹Fasteners must be installed a minimum of 4 inches from the top, bottom and end of the wall.

²See Section 3.2.2 for CMU, mortar and grout requirements.

³No more than one low-velocity fastener may be installed in an individual CMU cell.

⁴Fastener must be located a minimum of 1¹/₂ inches from the mortar joints, center web and end web of the CMU.

⁵Fasteners must not be installed in the head joints. Fasteners installed in the bed joints must be installed a minimum of 8 inches from the end of the wall.

Multiple fasteners in a bed joint must be spaced a minimum of 8 inches.

⁶Shear load can be in any direction.

⁷Shear direction must be horizontal along the CMU wall plane.

⁸Fastener located in center of grouted cell must be installed vertically.

⁹The fasteners listed in the table above may be used for static load conditions and for the seismic load condition described in Item 1 of Section 4.1.5.



X-W6

X-R FIGURE 1—HILTI FASTENERS

W10

X-CR##

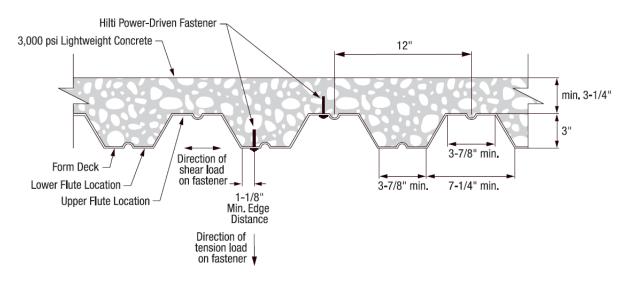


FIGURE 2—HILTI FASTENER LOCATIONS IN 3-INCH-DEEP COMPOSITE FLOOR DECK PANEL

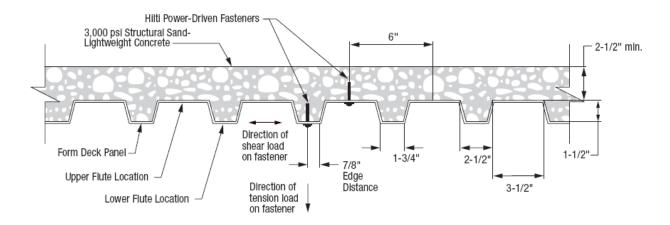
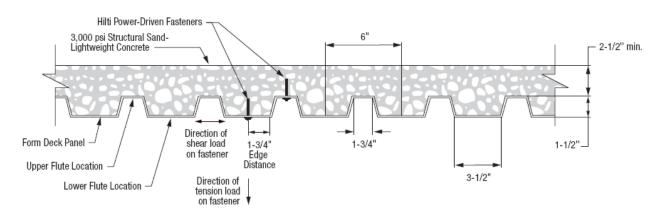


FIGURE 3—HILTI FASTENER LOCATIONS IN $1^{1}/_{2}$ -INCH-DEEP COMPOSITE FLOOR DECK PANEL, NORMAL DECK PANEL PROFILE ORIENTATION



For **SI:** 1 inch = 25.4 mm, 1 psi = 6895 Pa.

FIGURE 4—HILTI FASTENER LOCATIONS IN $1^1\!I_2$ -INCH-DEEP COMPOSITE FLOOR DECK PANEL, INVERTED DECK PANEL PROFILE ORIENTATION

TABLE 8—CODE SECTION NUMBER REFERENCE MATRIX

	IBC									
2024 IBC	2021 IBC	2018 IBC	2015 IBC							
Section 2.4 of ASCE 7-22	Section 2.4 of ASCE 7-16/S1	1605.3.1	1605.3.1							
(Referenced in IBC Section 1605.1)	(Referenced in IBC Section 1605.1)	1605.5.1	1605.5.1							
1605.2	1605.2	1605.3.2	1605.3.2							
1901.3	1901.3	1901.3	1901.3							
2107	2107	2107	2107							
2201.4	2204.1 and 2204.2	2204.1 and 2204.2	2204.1 and 2204.2							
2304.10.6.4	2304.10.6.4	2304.10.5.4	2304.10.5.4							
	AISI S100 Edition and Section	n Number								
2016(2020) w/S2-20 J5	2016(2020) w/S2-20 J5	2016 J5	2012 E5							
2016(2020) w/S2-20 J5.2	2016(2020) w/S2-20 J5.2	2016 J5.2	2012 E5.2							
2016(2020) w/S2-20 J5.2.1	2016(2020) w/S2-20 J5.2.1	2016 J5.2.1	2012 E5.2.1							
2016(2020) w/S2-20 J5.2.3	2016(2020) w/S2-20 J5.2.3	2016 J5.2.3	2012 E5.2.3							
2016(2020) w/S2-20 J5.3	2016(2020) w/S2-20 J5.3	2016 J5.3	2012 E5.3							
2016(2020) w/S2-20 J5.3.1	2016(2020) w/S2-20 J5.3.1	2016 J5.3.1	2012 E5.3.1							
2016(2020) w/S2-20 J5.3.2	2016(2020) w/S2-20 J5.3.2	2016 J5.3.2	2012 E5.3.2							
2016(2020) w/S2-20 J5.3.3	2016(2020) w/S2-20 J5.3.3	2016 J5.3.3	2012 E5.3.3							
2016(2020) w/S2-20 J5.3.4	2016(2020) w/S2-20 J5.3.4	2016 J5.3.4	2012 E5.3.4							
2016(2020) w/S2-20 J5.3.5	2016(2020) w/S2-20 J5.3.5	2016 J5.3.5	2012 E5.3.5							
	Other Referenced Stand	ards								
ASCE 7-22 Table 13.1-1	ASCE 7-16 w/S1 13.1.4	ASCE 7-16 13.1.4	ASCE 7-10 w/S1 13.1.4							
TMS 402-22 8.1.4	TMS 402-16 8.1.3	TMS 402-16 8.1.3	TMS 402-13 8.1.3							
	IRC									
2024 IRC	2021 IRC	2018 IRC	2015 IRC							
R301.1.3	R301.1.3	R301.1.3	R301.1.3							
R402.2	R402.2	R402.2	R402.2							



ICC-ES Evaluation Report

ESR-1663 City of LA Supplement

Reissued March 2025 Revised March 27, 2025

This report is subject to renewal March 2027.

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DIVISION: 03 00 00—CONCRETE

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DIVISION: 06 00 00—WOOD, PLASTICS AND COMPOSITES Section: 06 05 23—Wood, Plastic and Composite Fastenings

DIVISION: 09 00 00—FINISHES Section: 09 22 16.23—Fasteners

REPORT HOLDER:

HILTI, INC.

EVALUATION SUBJECT:

HILTI LOW-VELOCITY POWER-ACTUATED FASTENERS

1.0 REPORT PURPOSE AND SCOPE

Purpose:

The purpose of this evaluation report supplement is to indicate that Hilti Low-Velocity Power-Actuated Fasteners, described in ICC-ES evaluation report <u>ESR-1663</u>, have also been evaluated for compliance with the codes noted below as adopted by the Los Angeles Department of Building and Safety (LADBS).

Applicable code editions:

- 2023 City of Los Angeles Building Code (LABC)
- 2023 City of Los Angeles Residential Code (LARC)

2.0 CONCLUSIONS

The Hilti Low-Velocity Power-Actuated Fasteners, described in Sections 2.0 through 7.0 of the evaluation report <u>ESR-1663</u>, comply with the LABC Chapter 19, 21, 22, 23, and the LARC, and are subjected to the conditions of use described in this supplement.

3.0 CONDITIONS OF USE

The Hilti Low-Velocity Power-Actuated Fasteners described in this evaluation report supplement must comply with all of the following conditions:

- All applicable sections in the evaluation report ESR-1663.
- The design, installation, conditions of use and identification of the fasteners are in accordance with the 2021 *International Building Code*® (2021 IBC) provisions noted in the evaluation report <u>ESR-1663</u>.
- The design, installation and inspection are in accordance with additional requirements of LABC Chapters 16 and 17, as applicable.



- Under the LARC, an engineered design in accordance with LARC Section R301.1.3 must be submitted.
- The allowable strength values listed in the evaluation report ESR-1663 are for the connection of the fasteners to normalweight concrete, lightweight concrete with or without metal deck, steel, and masonry. The connection between the fasteners and the connected members must be checked for capacity (which may govern).

This supplement expires concurrently with the evaluation report, reissued March 2025, and revised March 27, 2025.



ICC-ES Evaluation Report

ESR-1663 FL Supplement w/ HVHZ

Reissued March 2025 Revised March 27, 2025

This report is subject to renewal March 2027.

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1.0 REPORT PURPOSE AND SCOPE

Purpose:

The purpose of this evaluation report supplement is to indicate that the Hilti Low-Velocity Power-Actuated Fasteners, addressed in ICC-ES evaluation report ESR-1663, have also been evaluated for compliance with the codes noted below.

Applicable code editions:

- 2023 Florida Building Code—Building
- 2023 Florida Building Code—Residential

2.0 CONCLUSIONS

The Hilti Low-Velocity Power-Actuated Fasteners, described in Sections 2.0 through 7.0 of the evaluation report ESR-1663, comply with the *Florida Building Code—Building* and the *Florida Building Code—Residential*, with the replacement of the referenced ASCE 7 edition as noted below. The design requirements shall be determined in accordance with the *Florida Building Code—Building* or the *Florida Building Code—Residential*, as applicable. The installation requirements noted in ICC-ES evaluation report ESR-1663 for the 2021 *International Building Code®* meet the requirements of the *Florida Building Code—Building* or the *Florida Building Code—Residential*, as applicable.

References to ASCE 7-16/S1 in ESR-1663 shall be replaced with ASCE 7-22. This is necessary as the 2023 Florida Building Code—Building references ASCE 7-22 and the 2021 International Building Code® references ASCE 7-16/S1.

Use of the Hilti Low-Velocity Power-Actuated Fasteners has also been found to be in compliance with the High-Velocity Hurricane Zone provisions of the *Florida Building Code—Building* and the *Florida Building Code—Residential* under the following conditions:

- The use of Hilti Low-Velocity Power-Actuated Fasteners as a means of attachment for wood blocking, as defined in Section 2330.1.10 of the Florida Building Code—Building, is prohibited.
- The fasteners have not been evaluated for use as cast-in-place anchors for compliance with the High-Velocity Hurricane Zone provisions, and this use is outside the scope of this supplement.



For products falling under Florida Rule 61G20-3, verification that the report holder's quality assurance program is audited by a quality assurance entity approved by the Florida Building Commission for the type of inspections being conducted is the responsibility of an approved validation entity (or the code official, when the report holder does not possess an approval by the Commission).

This supplement expires concurrently with the evaluation report, reissued March 2025, and revised March 27, 2025.